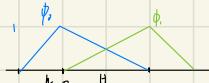
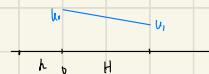




Element matrices



not evenly spaced interval



$$u(x) = u_0 \phi_0(x) + u_1 \phi_1(x)$$

weak form: $\int_0^1 C(x) \frac{du}{dx} \frac{dv}{dx} dx = \int_0^1 f(x) v(x) dx \quad \forall v(x)$

$$\int_0^1 C(x) \left(u_0 \phi_0'(x) + \dots + u_n \phi_n'(x) \right) \frac{du}{dx} dx = \int_0^1 f(x) V(x) dx$$

$$\int_0^1 C(x) \left(u_0 \phi_0'(x) + u_1 \phi_1'(x) \right) \frac{d\phi}{dx} dx \Rightarrow ku$$

$$\int_0^1 C(x) \left(u_0 \phi_0'(x) + u_1 \phi_1'(x) \right) \left(u_0 \phi_0'(x) + u_1 \phi_1'(x) \right) dx \Rightarrow U^T K U$$

$$\int_0^1 C(x) \left(\frac{du}{dx} \right)^2 dx = U^T K U$$

global K where $K_{ij} = \int_0^1 C(x) \frac{du_i}{dx} \frac{du_j}{dx} dx$

instead integrating $\phi'_i \cdot \phi'_j$ on the whole interval
integrate all ϕ'_i s on an element interval

$$\int_0^H C(x) \left(\frac{du}{dx} \right)^2 dx = U^T K_e U$$

$$\int_0^H \left(\frac{(u_i - u_0)^2}{H} \right)^2 dx = \frac{1}{H} (u_i - u_0)^2 \int_0^H C(x) dx \rightarrow [U_i, U_0]$$

$$C \underbrace{\begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}}_{\text{local } K_{ij}} \underbrace{\begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}}_{\text{local } U_i} \underbrace{\begin{bmatrix} u_i \\ u_0 \end{bmatrix}}_{\text{local } U}$$

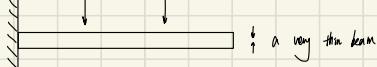
(take $C = C(H)$)

$$K = \frac{C_h}{h} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} + \frac{C_h}{H} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

K_{e1} K_{e2}

Simple idea in 1D . Key idea in 2D

4th order bending equation



fix-free beam (cantilever)

displacement $u(x)$

$$+ \text{boundary condition on } u: u(0)=0$$

+ boundary condition on u' : $u'(0)=0$ [fixed end]

curvature e

$$e = \frac{u''}{1+u'^2}$$

equilibrium equation $w=f(x)$

$$\ddot{A} = \frac{\partial f}{\partial x}$$

bending stiffness C

$$w=0 \quad w'=0 \quad \left. \begin{array}{l} \text{fixed end} \\ \text{free end} \end{array} \right\}$$

bending moment $w(x) = C(x) \cdot e(x)$

small curvature (u'') small compared to 1

equivalence of Hooke's law

$$\frac{\partial^2}{\partial x^2} \left(C(x) \cdot e(x) \right) = \frac{\partial^2}{\partial x^2} \left(C(x) \frac{\partial^2 u}{\partial x^2} \right) = f(x)$$

g. when $C(x)=1$ $f(x)=1$ $u''=1$

$$\left\{ \begin{array}{l} u'''=1 \\ u(0)=0 \\ u''(0)=w(0)=0 \\ u'(1)=0 \\ u''(1)=w(1)=0 \end{array} \right.$$

π simply supported
 π uniform load

$$u(x) = \frac{1}{24} x^4 + a + bx + cx^2 + dx^3$$

g. when $C(x)=1$ $f(x)=f(x-a)$

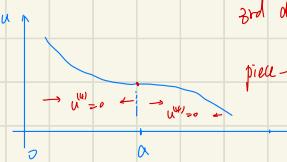
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$f(x-a)$

step function
quadratic
cubic $u_p = x^3/6$

$$u(x) = x^3/6 + C_1 + C_2 x + C_3 x^2$$

3rd derivative jumps



$u(x)$ is a cubic spline

